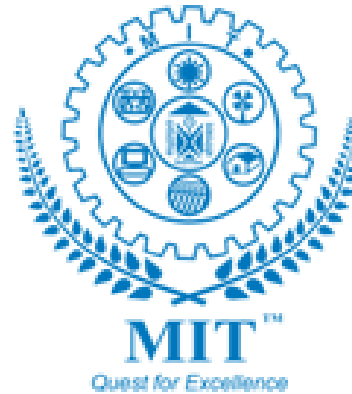


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Design of Flexible & Rigid Pavement

Contents:

B. Design of Rigid Pavement.

- *Design parameter.*
- *Westergaard Stress: Wheel Load, Temperature Stresses.*
- *Design of Joints in Rigid Pavement.*

Difference between Flexible & Rigid Pavement

<i>Sr. No.</i>		<i>Flexible Pavement</i>	<i>Rigid Pavement</i>
<i>1</i>	<i>IRC Code</i>	<ul style="list-style-type: none"><i>• IRC: 37</i>	<ul style="list-style-type: none"><i>• IRC: 58</i>
<i>2</i>	<i>Layers</i>	<ol style="list-style-type: none"><i>1. Surface Coarse (BC)</i><i>2. Base Coarse (WBM or WMM)</i><i>3. Subbase Coarse (GSB)</i><i>4. Subgrade (C-ϕ Soil).</i>	<ol style="list-style-type: none"><i>1. Surface Coarse (PCC or RCC)</i><i>2. Base Coarse (DLC or WMM)</i><i>3. Subgrade (C-ϕ Soil).</i>
<i>3</i>	<i>Load Transformation</i>	<ul style="list-style-type: none"><i>• Load Transformation is grain to grain.</i>	<ul style="list-style-type: none"><i>• Load Transformation is layer to layer.</i>
<i>4</i>	<i>Failure Nature</i>	<ul style="list-style-type: none"><i>• If there is any failure at bottom layer then the failure will be appear at top.</i>	<ul style="list-style-type: none"><i>• If there is any failure at bottom layer then for small cavity slab will act as a bridge over it.</i>
<i>5</i>	<i>Joints</i>	<ul style="list-style-type: none"><i>• Joints are absent.</i>	<ul style="list-style-type: none"><i>• Joints are present.</i>
<i>6</i>	<i>Cost</i>	<ul style="list-style-type: none"><i>• Initial cost is low but maintenance cost is high.</i>	<ul style="list-style-type: none"><i>• Initial cost is high but maintenance cost is low.</i>

Stresses in Rigid Pavement

1. Load Stress: Due to load.

2. Temperature stress:

- Warping stress: Due to day night temperature variation.*
- Frictional stress: Due to seasonal temperature variation.*

Design Parameter:

1. Modulus of subgrade reaction: (k)

• $k = (p/\Delta) = (p/0.125) \text{ kg/cm}^3$ $\Delta = 0.125 \text{ cm}$

Where, p = Contact pressure or tyre pressure in kg/cm^2

$$k = \text{kg/cm}^3$$

$$\Delta = 0.125 \text{ cm}$$

2. Radius of Relative stiffness: l (in cm) = $\left(\frac{Eh^3}{12k(1-\mu^2)}\right)^{1/4}$

3. Equivalent Radius of Resisting section: b (in cm)

- $b = \sqrt{1.6 a^2 + h^2} - 0.675 h$ *If $a/h \leq 1.724$*
- $b = a$ *If $a/h > 1.724$*
- *Effective in resisting bending moment.*
- *Radius of effective area (b), resisting bending moment.*
- $E = \text{Modulus of elasticity (Young's modulus) of slab (kg/cm}^2\text{)}$
- $h = \text{Thickness of slab or pavement (cm)}$
- $k = \text{Modulus of subgrade reaction (kg/cm}^3\text{)}$
- $\mu = \text{Poisson's ratio}$
- $a = \text{Radius of contact area (Radius of wheel load distribution) in (cm)}$

1. Load Stress: *Stresses developed in concrete slab due to wheel load.*

Westergaard stress equation for wheel load:

Magnitude,

a. At interior: S_i = Wheel load stress at Interior location in kg/cm²

$$S_i = \frac{0.316 P}{h^2} (4 \log_{10}(l/b) + 1.069)$$

b. At edge: S_e = Wheel load stress at edge location in kg/cm²

$$S_e = \frac{0.502 P}{h^2} (4 \log_{10}(l/b) + 0.359)$$

c. At corner: S_c = Wheel load stress at corner location in kg/cm²

$$S_c = \frac{3 P}{h^2} \left(1 - \left(\frac{a\sqrt{2}}{l} \right)^{0.6} \right)$$

Where,

P = Wheel load (kg)

l = Radius of relative stiffness (Pressure deformation characteristic of cement concrete) in (cm)

2. Warping stress or Temperature stresses: in kg/cm²

- Warping stress is developed due to day-night temperature variation. Under warping, effective temperature gradient exist between top and bottom of slab.*

Magnitude,

a. At Interior:

$$S (ti) = \frac{E * \alpha * T}{2} \left(\frac{Cx + \mu * Cy}{1 - \mu^2} \right)$$

b. At edge:

$$S (te) = \frac{Cx * E * \alpha * T}{2} \text{ OR } \frac{Cy * E * \alpha * T}{2} \dots\dots\dots \text{Take maximum value from both}$$

c. At corner:

$$S (tc) = \frac{E * \alpha * T}{3 (1 - \mu)} \sqrt{\frac{a}{l}}$$

Where,

α = Coefficient of thermal expansion (per °c)

T = Change in temperature (Temperature difference between top and bottom of pavement surface) in (°c)

- C_x & C_y are the factor (Warping stress coefficient) which depends upon L_x/l & L_y/l respectively.
- L_x = length of slab (Spacing between two transverse joint)
- L_y = Width of slab (Spacing between two longitudinal joint)

3. Frictional Stress (S_f): Due to seasonal temperature variation in kg/cm^2

- Frictional force developed by half of slab is resisted by tensile force at center.

- $$S_f = \frac{f * \gamma * L}{2 * 10^4}$$

Where,

- f = Coefficient of friction between pavement surface and subgrade soil.
- L = Length of pavement section.
- γ = Density = Weight/Volume in kg/cm^3

- *Frictional resistance at corner position is always zero.*
- *In summer season frictional stresses are developed which are compressive in nature.*

Critical Combination of stresses:

1. Critical combination for edge & interior location when pavement is constructed in summer.

- *$I \text{ \& } E \text{ (in Summer)} = \text{Wheel load stress} + \text{Temperature stress} - \text{frictional stress}$*
- *$\text{Interior} = S_i + S(t_i) - S_f$*
- *$\text{Edge} = S_e + S(t_e) - S_f$*

2. Critical combination for edge & interior location when pavement is constructed in winter.

- *$I \text{ \& } E \text{ (in Winter)} = \text{Wheel load stress} + \text{Temperature stress} + \text{frictional stress}$*

3. Critical combination of stresses at corner location.

- *$\text{Corner} = \text{Wheel load stress} + \text{Temperature stress}$*

Question:

- *A pavement slab 22 cm thick is constructed over a soil subgrade having modulus of subgrade reaction of 18 kg/cm^3 . Spacing between transverse joints & longitudinal joints are 5.5 m & 4.2 m respectively. Find out the stresses due to load, temperature & worst combination of stresses by using the following data.*
- *Wheel load (P)= 4500 kg*
- *Temperature difference (T)= 20°C*
- *Radius of load contact (a)= 15 cm*
- *Elastic modulus of cement concrete (E) = $3 * 10^5 \text{ kg/cm}^2$*
- *Poisson's ratio (μ) = 0.15*
- *Coefficient of friction (f) = 1.5*
- *Coefficient of thermal expansion (α) = $12 * 10^{-6}$ per $^\circ\text{C}$*
- *$C_x = 1.05$ & $C_y = 0.9$*

Answer:

Given, $h = 22 \text{ cm}$, $k = 18 \text{ kg/cm}^3$, $Lx = 5.5 \text{ m}$, $Ly = 4.2 \text{ m}$

Step 1: Design parameter

$$k = p/0.125 = \frac{P/\mu a^2}{0.125} = \frac{4500/\mu * 15^2}{0.125} = \mathbf{18 \text{ kg/cm}^3}$$

$$l = \left(\frac{Eh^3}{12k(1-\mu^2)} \right)^{1/4} = \left(\frac{3 * 10^5 * 22^3}{12 * 18(1-0.15^2)} \right)^{1/4}$$

$$l = \mathbf{62.36 \text{ cm}}$$

$$b = \sqrt{1.6 a^2 + h^2} - 0.675 h = \sqrt{1.6 * 15^2 + 22^2} - 0.675 * 22$$

$$b = \mathbf{14.20 \text{ cm}}$$

Step 2: Stresses

1. Wheel load stresses: (in kg/cm²)

$$a. \quad Si = \frac{0.316 P}{h^2} (4 \log_{10}(l/b) + 1.069) = \frac{0.316 * 4500}{22^2} (4 \log_{10}(62.36/14.20) + 1.069)$$

$$Si = 10.69 \text{ kg/cm}^2$$

$$b. \quad Se = \frac{0.502 P}{h^2} (4 \log_{10}(l/b) + 0.359) = \frac{0.502 * 4500}{22^2} (4 \log_{10}(62.36/14.20) + 0.359)$$

$$Se = 15.57 \text{ kg/cm}^2$$

$$c. \quad \text{At Corner, } Sc = \frac{3 P}{h^2} (1 - (\frac{a\sqrt{2}}{l})^{0.6}) = \frac{3 * 4500}{22^2} (1 - (\frac{15\sqrt{2}}{62.36})^{0.6})$$

$$Sc = 13.28 \text{ kg/cm}^2$$

2. *Warping Stresses: (in kg/cm²)*

$$a. S (ti) = \frac{E * \alpha * T}{2} \left(\frac{Cx + \mu * Cy}{1 - \mu^2} \right) = \frac{3 * 10^5 * 12 * 10^{-6} * 20}{2} \left(\frac{1.05 + 0.15 * 0.9}{1 - 0.15^2} \right)$$

$$S (ti) = \mathbf{43.64 \text{ kg/cm}^2}$$

$$b. S (te) = \frac{Cx * E * \alpha * T}{2} \text{ OR } \frac{Cy * E * \alpha * T}{2}$$

$$S (te) = \frac{Cx * E * \alpha * T}{2} = \frac{1.05 * 3 * 10^5 * 12 * 10^{-6} * 20}{2}$$

$$S (te) = \mathbf{37.8 \text{ kg/cm}^2}$$

$$c. \text{ At Corner, } S (tc) = \frac{E * \alpha * T}{3 (1 - \mu)} \sqrt{\frac{a}{l}} = \frac{3 * 10^5 * 12 * 10^{-6} * 20}{3 (1 - 0.15)} \sqrt{\frac{15}{62.36}}$$

$$S (tc) = \mathbf{13.84 \text{ kg/cm}^2}$$

3. Frictional Stresses:

$$S_f = \frac{f * \gamma * L}{2 * 10^4} = \frac{1.5 * 2400 * 5.5}{2 * 10^4} = \mathbf{0.99 \text{ kg/cm}^2}$$

Step 3: Worst combination of stresses: (in kg/cm²)

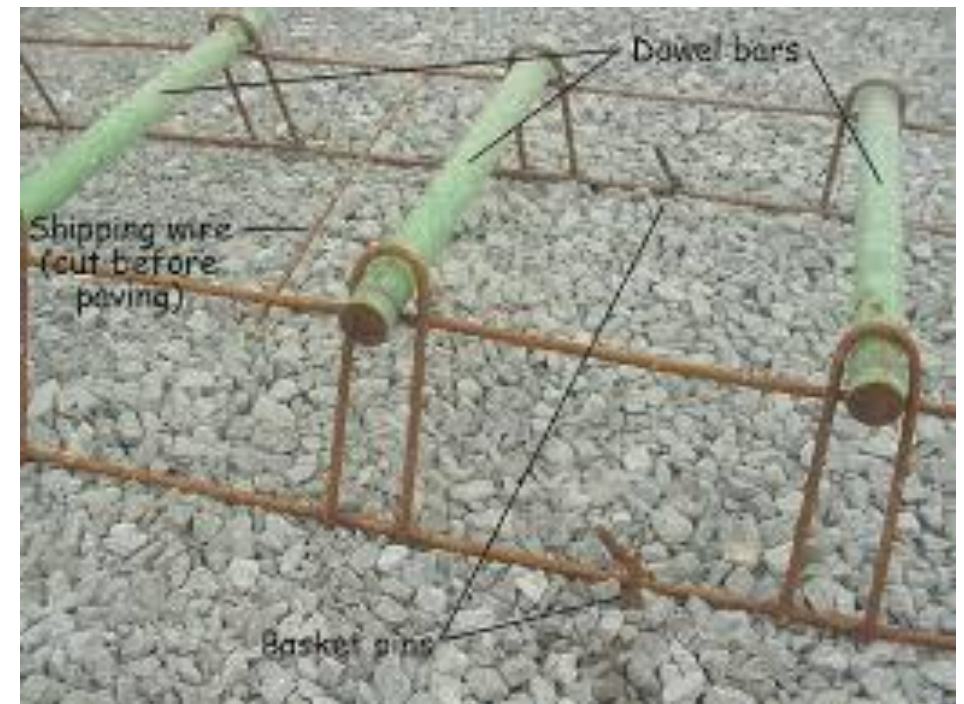
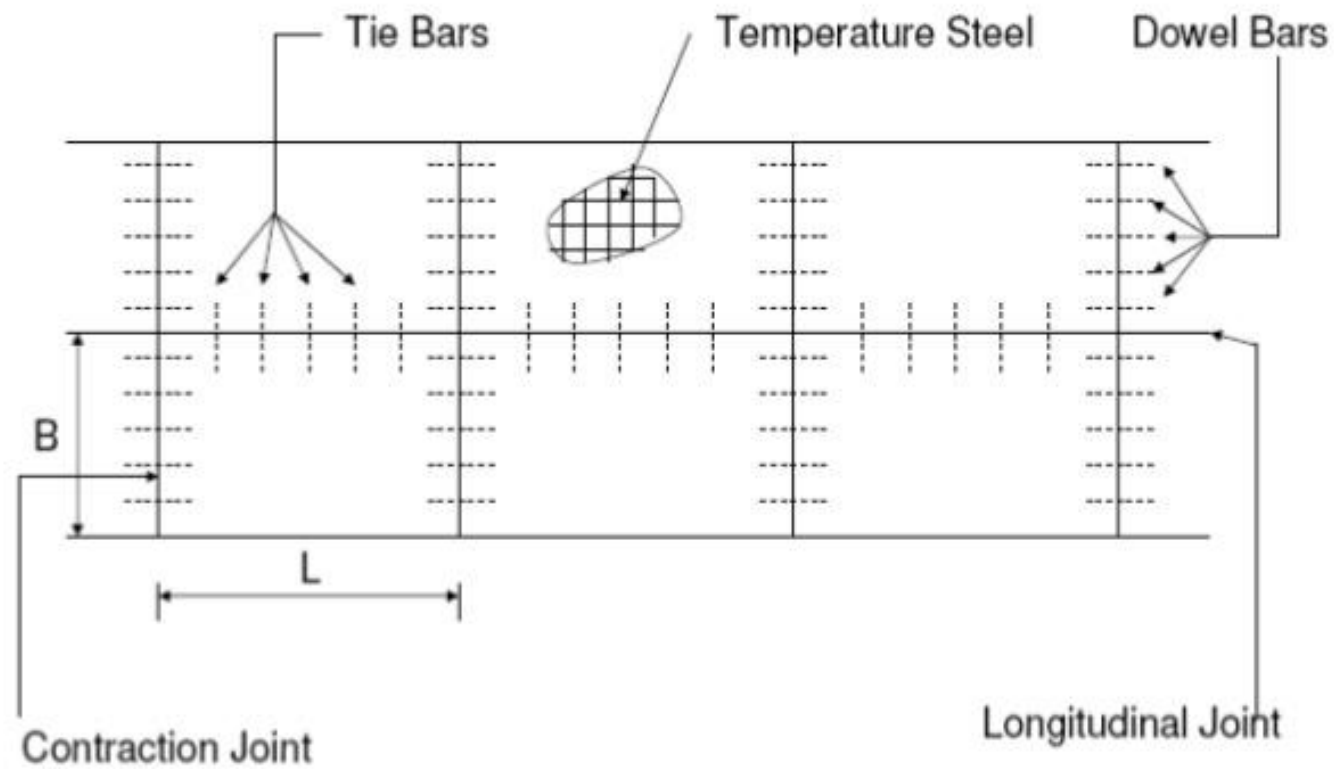
- *In Summer, (At Interior) : $S(I) = S_i + S(ti) - S_f = 10.69 + 43.64 - 0.99 = 53.34$*
(At edge) : $S(E) = S_e + S(te) - S_f = 15.57 + 37.8 - 0.99 = 52.38$
- *In Winter, (At Interior) : $S(I) = S_i + S(ti) + S_f = 10.69 + 43.64 + 0.99 = 55.32$*
(At edge) : $S(E) = S_e + S(ti) + S_f = 15.57 + 37.8 + 0.99 = 54.36$
- *At Corner, $S(C) = S_c + S(tc) = 13.28 + 13.84 = 27.12$*

Design of Joints in Rigid Pavement:

- 1. Transverse joints: Provided in transverse direction of slab.*
 - a. Contraction Joints*
 - b. Expansion Joints*
- 2. Longitudinal Joints: Provided in longitudinal direction of slab.*



Jointed CC Pavement



- *Contraction & expansion joints are provided in rigid pavement due to temperature variation. (Increases or decreases)*

Contraction Joints:

- *Provided in transverse joints to allow contraction of slab section with respect to construction temperature & decrease in temperature of slab.*

Case 1: When PCC used (Without Dowel bar)

- *Frictional force generated by half of slab = Resisting force by concrete at center*

- $f \left(\gamma * \frac{L}{2} * B * h \right) = Sf * B * h$

- $L = \frac{2 * Sf}{\gamma * f}$

- γ = Density of concrete in kg/m³

- f = Coefficient of subgrade restrained = 1.5

- Sf = Allowable stress in tension in cement concrete in kg/cm²

- *Spacing between two contraction joints:*

- $L = \frac{2 * Sf}{\gamma * f} * 10^4$

As per IRC,

- *The spacing between two contraction joint for PCC is ≤ 4.5 m*

Case 2: When Dowel bar used.

- *Frictional force generated by half of slab = Resisting force by steel*

- $f \left(\gamma * \frac{L}{2} * B * h \right) = \sigma(st) * A(st)$

- $$L = \frac{2 * \sigma(st) * A(st)}{\gamma * f * B * h}$$

Where,

- $\sigma(st)$ = *Allowable tensile stress in steel*
- $A(st)$ = *Area of steel provided for entire width of the pavement in cm^2*

Question:

- *Design the spacing between two contraction joint if maximum tensile stress of concrete is 0.8 kg/cm^2 and coefficient of friction is 1.5 (Assume $\gamma = 2400 \text{ kg/m}^3$)*

Answer:

- $L = \frac{2 * S_f}{\gamma * f} * 10^4 = \frac{2 * 0.8}{2400 * 1.5} * 10^4$
- $L = 4.44 \text{ m} \leq 4.5 \text{ m}$ (Safe)

Question:

- *Calculate the spacing between contraction joints by using the following data.*
- *Width of slab (B) = 4.52 m*
- *Thickness of slab (h) = 25 cm*
- *Coefficient of friction (f) = 1.5*
- *Diameter of bar (ϕ) = 12 mm*
- *Spacing between the bar = 300 mm*
- *Allowable tensile stress in steel ($\sigma(st)$) = 1400 kg/cm^2*

Answer:

- *Number of bar* = $n = \frac{\text{Width of slab}}{\text{Spacing}} = \frac{4.52 * 10^3}{300} = 15.06 \approx 16 \text{ bars.}$
- $A(st) = n * \frac{\pi}{4} * \phi^2 = 16 * \frac{\pi}{4} * 12^2$
- $A(st) = 1809 \text{ mm}^2 = 18.09 \text{ cm}^2$
- *Spacing between contraction joint:*
- $L = \frac{2 * \sigma(st) * A(st)}{\gamma * f * B * h} = \frac{2 * 1400 * 18.09}{2400 * 1.5 * 4.52 * 0.25}$
- $L = 12.45 \text{ m}$

Expansion Joints:

- *Provided in transverse direction to allow expansion of slab in longitudinal direction.*
- *As per IRC, the gap of expansion joint should be designed such that even after full expansion of pavement 50 % of gap should be maintained.*
- *Allowable expansion = $(\Delta/4 + \Delta/4) = \Delta/2 = L\alpha T$*
- *$L = \frac{\Delta}{2\alpha T}$ = Spacing between expansion joints.*

Where,

- *Δ = Gap of expansion joints. (Maximum expansion in slab section)
= Half of the gap provided for joint 2.5/2*
- *α = Coefficient of thermal expansion*
- *T = Change in temperature*

As per IRC,

- 1. Filler material for expansion joint must have 50 % compressibility in order to allow expansion of slab. (e.g. fiber board, cork)*
- 2. The maximum gap provided, $\Delta \leq 2.5 \text{ cm}$*
- 3. The maximum spacing between expansion joint, $L \leq 140 \text{ m}$*
 - Expansion joint provided to allow the expansion of pavement due to rise in temperature with respect to construction temperature.*
 - Dowel bars are provided only in expansion joint.*
 - The system of reinforcement transfer certain amount of load to next section is known as dowel bars.*

Question:

- If the width of expansion joint is 2 cm & temperature at the time of construction of slab is 20°C , then find the spacing between expansion joints for a maximum temperature of 45°C . (Take $\alpha = 12 * 10^{-6} / ^{\circ}\text{C}$)*

Answer:

• *Given,*

$$\Delta = 2 \text{ cm} = 2 * 10^{-2} \text{ m}$$

$$T = 45 - 20 = 25 \text{ }^{\circ}\text{C}$$

$$\alpha = 12 * 10^{-6} /^{\circ}\text{C}$$

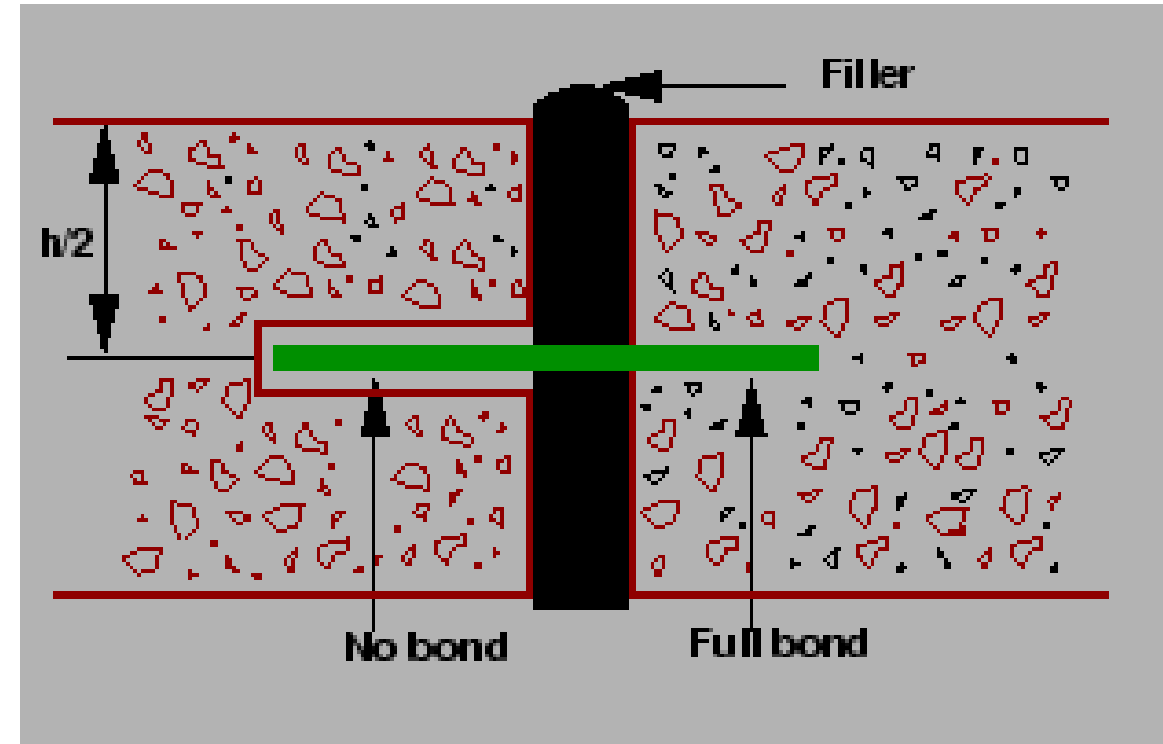
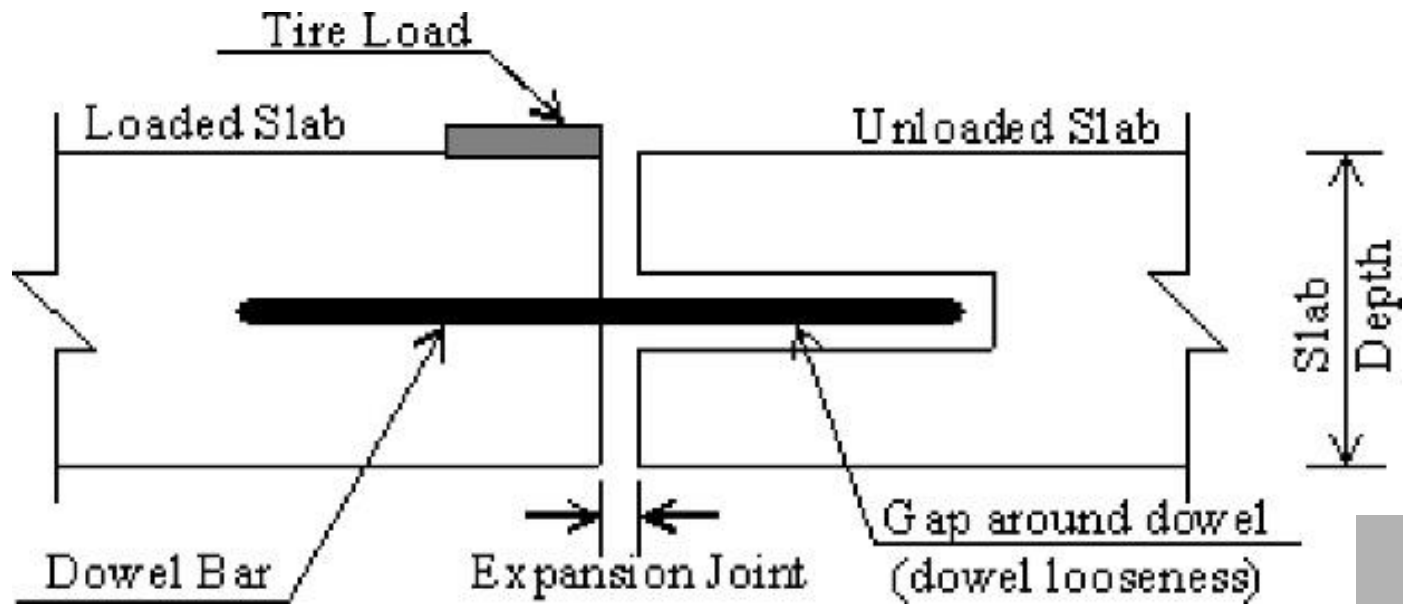
$$L = ?$$

$$\bullet L = \frac{\Delta}{2\alpha T} = \frac{2 * 10^{-2}}{2 * 12 * 10^{-6} * 25} = 33.333 < 140 \text{ m (Safe)}$$

Purpose to provide dowel bar:

- *To transfer wheel load from one slab to another slab.*
- *To reduce the differential deflection.*

- *Arrangement of Dowel bar at expansion joint:*



Longitudinal joints:

- *Provided longitudinally along the road to account lateral expansion & contraction.*
- *Tie bars used at longitudinal joint to restrain slab against lateral sliding under dynamic effect of load due to vehicle movements expansion and contraction.*

Design of tie bars:

- *Frictional force generated by 1 m of slab = Resistive force by steel per (m) slab.*
- $f * (\gamma * B * 1 * h) = \sigma(st) * A(st)$
- $A(st) = \frac{f * (\gamma * B * h)}{\sigma(st)}$ = Area of steel in cm² required per metre slab.

Where, $\sigma(st) = \text{kg/cm}^2$

- *$B = \text{Distance between free edge of the pavement and longitudinal joint.}$*
- $B = \frac{\text{Width of pavement}}{2}$, h in cm, $f = 1.5$, $\gamma = \text{kg/m}^3$

- Assume, diameter of tie bar = ϕ
- Number of tie bars per metre slab = $n = \frac{\text{Total area}}{\text{Area of 1 bar}} = \frac{A(st)}{\left(\frac{\pi}{4}\right) * \phi^2}$
- Spacing between tie bar = $S = 1000/n$

As per IRC,

- ϕ = Diameter of bar ≤ 20 mm
- S = Spacing of bar ≤ 75 cm

Length of Tie bars (L_T):

- Bond force at steel per m slab length = Tensile force in steel per (m) slab length.

$$\tau_{bd} * \left(n * \frac{L_T}{2} * \pi * \phi\right) = \sigma(st) * \left(n * \frac{\pi}{4} * \phi^2\right)$$

$$L_T = \frac{\sigma(st) * \phi}{2 * \tau_{bd}}$$

Where,

- τ_{bd} = *Permissible bond stress of concrete in kg/cm²*
- Φ = *Diameter of tie bar in cm*

Question:

- *Cement concrete pavement has a thickness of 24 cm & has two lane of width 7.2 m with a longitudinal joint in between. Design the dimension & spacing of the bar by using following data.*
- *Allowable tensile stress in steel = 1400 kg/cm²*
- *Unit weight of cement concrete = 2400 kg/cm³*
- *Coefficient of friction = $f = 1.5$*
- *Bond stress between steel bar and concrete = 24.6 kg/cm²*

Answer:

- *Given, $f = 1.5$, $\sigma(st) = 1400 \text{ kg/cm}^2$, $\gamma = 2400 \text{ kg/cm}^3$, $B = (7.2/2) = 3.6 \text{ m}$
 $h = 24 \text{ cm} = 0.24 \text{ m}$, $\tau_{bd} = 24.6 \text{ kg/cm}^2$*
- *Area of steel required per metre slab*
- $A(st) = \frac{f * (\gamma * B * h)}{\sigma(st)} = \frac{1.5 * 2400 * 3.6 * 0.24}{1400} = 2.22 \text{ cm}^2 = 222 \text{ mm}^2$
- *Assume, $\phi = 10 \text{ mm}$*
- $n = \frac{\text{Total area}}{\text{Area of 1 bar}} = \frac{A(st)}{\left(\frac{\pi}{4}\right) * \phi^2} = \frac{222}{\frac{\pi}{4} * 10^2} = 2.82 \approx 3 \text{ number}$
- *Spacing, $S = 1000/n = 1000/3 = 333.33 \text{ mm} = 33.33 \text{ cm} \leq 75 \text{ cm}$ (Safe)*
- $\text{Length of Tie bar} = L_T = \frac{\sigma(st) * \phi}{2 * \tau_{bd}} = \frac{10 * 1400}{2 * 24.6} = 284.55 \text{ mm} = \mathbf{28.49 \text{ cm}}$

Thank You